

**SIXTH SEMESTER B.TECH. (ENGINEERING) DEGREE  
EXAMINATION, JUNE 2010**

CE 04 603—STRUCTURAL DESIGN—II

(2004 Admissions)

Time : Three Hours

Maximum : 100 Marks

*All designs shall be done as per IS : Specification.*

*S.I. units shall be followed.*

*Use of IS : 800, IS : 883, IS : 875 and SP 6 shall be permitted in the examination hall.*

- I. (a) What do you understand by semi-rigid connections ?  
 (b) What are the disadvantages of bolted connections ?  
 (c) What are the design criteria followed in the design of laterally restrained simple beams ?  
 (d) What are the design considerations in bearings ?  
 (e) Explain why the joint design in roof truss is important.  
 (f) Write a short note on slab base.  
 (g) What are the deflection considerations for the design of timber structures ?  
 (h) Mention the different loads to be considered in the design of roof trusses.

(8 × 5 = 40 marks)

- II. (a) Design a suitable longitudinal fillet weld to connect plates shown in below fig. and to transmit a pull equal to full strength of thin plate. Allowable stress in weld is  $110 \text{ N/mm}^2$  and tensile stress in plates is  $0.6 f_y$ . Plates of 10 mm thick.  $f_y = 250 \text{ N/mm}^2$ .

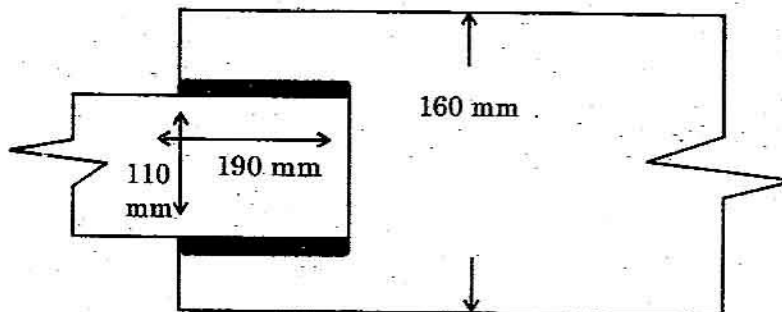


Fig. 1.

(15 marks)

Or

Turn over

- (b) A double riveted double cover butt joint is used to connect plates 10 mm thick. Determine diameter of rivet, rivet value, gauge and efficiency of joint. Adopt the following stress :

Working stresses in shear in power driven rivets =  $110 \text{ N/mm}^2$

Working stresses in bearing in power driven rivets =  $290 \text{ N/mm}^2$

For plates working stress in axial tension is  $0.6 f_y$  where  $f_y = 250 \text{ N/mm}^2$ .

(15 marks)

- III. (a) A steel column 12 m long carries an axial load of 1200 kN. Column is fixed at both ends. Design an economical built up section with double lacing. Design the lacing also.

(15 marks)

Or

- (b) ISMB 550 at  $1.037 \text{ kN/m}$  has been used as simply supported beam over a span of 5 m. Ends of beam are restrained against torsion but not against lateral bending. Determine the safe u.d.l. which the beam can carry.

(15 marks)

- IV. (a) A column section ISMB 300 at  $0.63 \text{ kN/m}$  with one cover plate  $375 \text{ mm} \times 22 \text{ mm}$  on either side is carrying an axial load of 2875 kN including self wt. of base and column. Design a Gusseted base. The allowable bending pressure in concrete is 4 MPa. The allowable stress in base plate is 185 MPa.

(15 marks)

Or

- (b) Design an angle iron purlin for a trussed roof from the following data :—

Span of roof truss = 1.5 m

Spacing of roof truss = 5 m

Spacing of purlins along slop of roof truss = 2.3 m

Slope of roof truss = 1 vertical to 2.25 horizontal

Wind load on roof normal to roof =  $1200 \text{ N/m}^2$

Vertical load from roof sheeting etc =  $250 \text{ N/m}^2$

- V. (a) A deodar timber beam carries u.d.l. (of  $0.7 \text{ kN/m}$  inclusive of self wt. of the beam. The beam is simply supported at both ends. The clear span of the beam is 6 m. Design the timber beam.

(15 marks)

Or

- (b) A beam is simply supported at its both the ends. The effective span is 5m. It consists of 200 mm × 300 mm deodar wood with 275 × 10 mm steel plates to its bids as shown below. Design the safe u.d.l. for the beam.

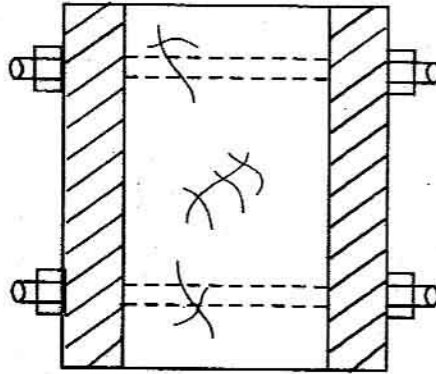


Fig. 2.

(15 marks)

[4 × 15 = 60 marks]